SECTION 1801
GENERAL

1801.1 Scope. The provisions of this chapter shall apply to building and foundation systems in those areas not subject to scour or water pressure by wind and wave action. Buildings and foundations subject to such scour or water pressure loads shall be designed in accordance with Chapter 16.

1801.2 Design. Allowable bearing pressures, allowable stresses and design formulas provided in this chapter shall be used with the allowable stress design load combinations specified in Section 1605.3. The quality and design of materials used structurally in excavations, footings and foundations shall conform to the requirements specified in Chapters 16, 19, 21, 22 and 23 of this code. Excavations and fills shall also comply with Chapter 33.

1801.2.1 Foundation design for seismic overturning. Where the foundation is proportioned using the strength design load combinations of Section 1605.2, the seismic overturning moment need not exceed 75 percent of the value computed from Section 1617.4.5 for the equivalent lateral force method, or Section 1618 for the modal analysis method.

SECTION 1802
FOUNDATION AND SOILS INVESTIGATIONS

1802.1 General. Foundation and soils investigations shall be conducted in conformance with Sections 1802.2 through 1802.6. Where required by the building official, the classification and investigation of the soil shall be made by a registered design professional.

1802.2 Where required. The owner or applicant shall submit a foundation and soils investigation to the building official where required in Sections 1802.2.1 through 1802.2.7.

Exception: The building official need not require a foundation or soils investigation where satisfactory data from adjacent areas is available that demonstrates an investigation is not necessary for any of the conditions in Sections 1802.2.1 through 1802.2.6.

1802.2.1 Questionable soil. Where the safe-sustaining power of the soil is in doubt, or where a load-bearing value superior to that specified in this code is claimed, the building official shall require that the necessary investigation be made. Such investigation shall comply with the provisions of Sections 1802.4 through 1802.6.

1802.2.2 Expansive soils. In areas likely to have expansive soil, the building official shall require soil tests to determine where such soils do exist.

1802.2.3 Groundwater table. A subsurface soil investigation shall be performed to determine whether the existing groundwater table is above or within 5 feet (1524 mm) below the elevation of the lowest floor level where such floor is located below the finished ground level adjacent to the foundation.

Exception: A subsurface soil investigation shall not be required where waterproofing is provided in accordance with Section 1806.

1802.2.4 Pile and pier foundations. Pile and pier foundations shall be designed and installed on the basis of a foundation investigation and report as specified in Sections 1802.4 through 1802.6 and Section 1807.2.1.

1802.2.5 Rock strata. Where subsurface explorations at the project site indicate variations or doubtful characteristics in the structure of the rock upon which foundations are to be constructed, a sufficient number of borings shall be made to a depth of not less than 10 feet (3048 mm) below the level of the foundations to provide assurance of the soundness of the foundation bed and its load-bearing capacity.

1802.2.6 Seismic Design Category C. Where a structure is determined to be in Seismic Design Category C in accordance with Section 1616, an investigation shall be conducted, and shall include an evaluation of the following potential hazards resulting from earthquake motions: slope instability, liquefaction, and surface rupture due to faulting or lateral spreading.

1802.2.7 Seismic Design Category D, E or F. Where the structure is determined to be in Seismic Design Category D, E or F, in accordance with Section 1616, the soils investigation requirements for Seismic Design Category C, given in Section 1802.2.6, shall be met, in addition to the following. The investigation shall include:

1. A determination of lateral pressures on basement and retaining walls due to earthquake motions.
2. An assessment of potential consequences of any liquefaction and soil strength loss, including estimation
of differential settlement, lateral movement or reduction in foundation soil-bearing capacity, and shall discuss mitigation measures. Such measures shall be given consideration in the design of the structure and can include, but are not limited to, ground stabilization, selection of appropriate foundation type and depths, selection of appropriate structural systems to accommodate anticipated displacements, or any combination of these measures. The potential for liquefaction and soil strength loss shall be evaluated for site peak ground acceleration magnitudes and source characteristics consistent with the design earthquake ground motions. Peak ground acceleration shall be determined from a site-specific study taking into account soil amplification effects, as specified in Section 1615.2.

**Exception:** A site-specific study need not be performed provided that peak ground acceleration equal to $S_{DS}/2.5$ is used, where $S_{DS}$ is determined in accordance with Section 1615.2.1.

### 1802.3 Soil classification
Where required, soils shall be classified in accordance with Section 1802.3.1 or 1802.3.2.

#### 1802.3.1 General
For the purposes of this chapter, the definition and classification of soil materials for use in Table 1804.2 shall be in accordance with ASTM D 2487.

#### 1802.3.2 Expansive soils
Soils meeting all four of the following provisions shall be considered expansive, except that tests to show compliance with Items 1, 2 and 3 shall not be required if the test prescribed in Item 4 is conducted:

1. Plasticity Index (PI) of 15 or greater, determined in accordance with ASTM D 4318.
2. More than 10 percent of the soil particles pass a No. 200 sieve (75 µm), determined in accordance with ASTM D 422.
3. More than 10 percent of the soil particles are less than 5 micrometers in size, determined in accordance with ASTM D 422.
4. Expansion Index greater than 20, determined in accordance with UBC Standard 18-2 or SBCCI SSTD 7.

#### 1802.4 Investigation
Soil classification shall be based on observation and any necessary tests of the materials disclosed by borings, test pits or other subsurface exploration made in appropriate locations. Additional studies shall be made as necessary to evaluate slope stability, soil strength, position and adequacy of load-bearing soils, the effect of moisture variation on soil-bearing capacity, compressibility, liquefaction and expansiveness.

### 1802.4.1 Exploratory boring
The scope of the soil investigation including the number and types of borings or soundings, the equipment used to drill and sample, the in-situ testing equipment, and the laboratory testing program shall be determined by a registered design professional.

#### 1802.5 Soil boring and sampling
The soil boring and sampling procedure and apparatus shall be in accordance with generally accepted engineering practice. The registered design professional shall have a fully qualified representative on the site during all boring and sampling operations.

#### 1802.6 Reports
The soil classification and design load-bearing capacity shall be shown on the construction document. Where required by the building official, a written report of the investigation shall be submitted that shall include, but need not be limited to, the following information:

1. A plot showing the location of test borings and/or excavations.
2. A complete record of the soil samples.
3. A record of the soil profile.
4. Elevation of the water table, if encountered.
5. Recommendations for foundation type and design criteria, including but not limited to: bearing capacity of natural or compacted soil; provisions to mitigate the effects of expansive soils; mitigation of the effects of liquefaction, differential settlement, and varying soil strength; and the effects of adjacent loads.
7. Pile and pier foundation information in accordance with Section 1807.2.1.
8. Special design and construction provisions for footings or foundations founded on expansive soils, as necessary.
9. Compacted fill material properties and testing in accordance with Section 1803.4.

### SECTION 1803
**EXCAVATION, GRADING AND FILL**

#### 1803.1 Excavations near footings or foundations
Excavations for any purpose shall not remove lateral support from any footing or foundation without first underpinning or protecting the footing or foundation against settlement or lateral translation.

#### 1803.2 Placement of backfill
The excavation outside the foundation shall be backfilled with soil that is free of organic material, construction debris, cobbles and boulders. The backfill shall be placed in lifts and compacted in a manner that does not damage the foundation or the waterproofing or dampproofing material.
1803.3 Site grading. The ground immediately adjacent to the foundation shall be sloped away from the building at a slope of not less than one unit vertical in 20 units horizontal (5-percent slope) for a minimum distance of 10 feet (3048 mm) measured perpendicular to the face of the wall or an approved alternate method of diverting water away from the foundation shall be used.

**Exception:** Where climatic or soil conditions warrant, the slope of the ground away from the building foundation is permitted to be reduced to not less than one unit vertical in 48 units horizontal (2-percent slope).

The procedure used to establish the final ground level adjacent to the foundation shall account for additional settlement of the backfill.

1803.4 Compacted fill material. Where footings will bear on compacted fill material, the compacted fill shall comply with the provisions of an approved report. The report shall contain the following:

1. Specifications for the preparation of the site prior to placement of compacted fill material.
2. Specifications for material to be used as compacted fill.
3. Test method to be used to determine the maximum dry density and optimum moisture content of the material to be used as compacted fill.
4. Maximum allowable thickness of each lift of compacted fill material.
5. Field test method for determining the in-place dry density of the compacted fill.
6. Minimum acceptable in-place dry density expressed as a percentage of the maximum dry density determined in accordance with Item 3.
7. Number and frequency of field tests required to determine compliance with Item 6.

**Exception:** Compacted fill material less than 12 inches (305 mm) in depth need not comply with an approved report, provided it has been compacted to a minimum of 90 percent Modified Proctor in accordance with ASTM D 1557. The compaction shall be verified by a qualified inspector approved by the building official.

SECTION 1804
ALLOWABLE LOAD-BEARING VALUES OF SOILS

1804.1 Design. The presumptive load-bearing values provided in Table 1804.2 shall be used with the allowable stress design load combinations specified in Section 1605.3.

1804.2 Presumptive load-bearing values. The maximum allowable foundation pressure, lateral pressure or lateral sliding resistance values for supporting soils at or near the surface shall not exceed the values specified in Table 1804.2 unless data to substantiate the use of a higher value are submitted and approved.

Presumptive load-bearing values shall apply to materials with similar physical characteristics and dispositions.

Mud, organic silt, organic clays, peat or unprepared fill shall not be assumed to have a presumptive load bearing capacity unless data to substantiate the use of such a value are submitted.

**Exception:** A presumptive load-bearing capacity is permitted to be used where the building official deems the load-bearing capacity of mud, organic silt or unprepared fill is adequate for the support of lightweight and temporary structures.

1804.3 Lateral sliding resistance. The resistance of structural walls to lateral sliding shall be calculated by combining the values derived from the lateral bearing and the lateral sliding resistance shown in Table 1804.2 unless data to substantiate the use of higher values are submitted for approval.

For clay, sandy clay, silty clay and clayey silt, in no case shall the lateral sliding resistance exceed one-half the dead load.

1804.3.1 Increases in allowable lateral sliding resistance. The resistance values derived from the table are permitted to be increased by the tabular value for each additional foot (305 mm) of depth to a maximum of 15 times the tabular value.

Isolated poles for uses such as flagpoles or signs and poles used to support buildings that are not adversely affected by a 1/8-inch (12.7 mm) motion at the ground surface due to short-term lateral loads are permitted to be designed using lateral-bearing values equal to two times the tabular values.

SECTION 1805
FOOTINGS AND FOUNDATIONS

1805.1 General. Footings and foundations shall be designed and constructed in accordance with Sections 1805.1 through 1805.9.

Footings and foundations shall be built on undisturbed soil or compacted fill material. Compacted fill material shall be placed in accordance with Section 1803.4.

The top surface of footings shall be level. The bottom surface of footings are permitted to have a slope not exceeding 1 unit vertical in 10 units horizontal (10-percent slope). Footings shall be stepped where it is necessary to change the elevation of the top surface of the footing or where the surface of the ground slopes more than 1 unit vertical in 10 units horizontal (10-percent slope).
### TABLE 1804.2
#### ALLOWABLE FOUNDATION AND LATERAL PRESSURE

<table>
<thead>
<tr>
<th>CLASS OF MATERIALS</th>
<th>ALLOWABLE FOUNDATION PRESSURE (psf)&lt;sup&gt;a&lt;/sup&gt;</th>
<th>LATERAL BEARING (psf/ft below natural grade)&lt;sup&gt;b&lt;/sup&gt;</th>
<th>LATERAL SLIDING</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Crystalline bedrock</td>
<td>12,000</td>
<td>1,200</td>
<td>0.70</td>
</tr>
<tr>
<td>2. Sedimentary and foliated rock</td>
<td>4,000</td>
<td>400</td>
<td>0.35</td>
</tr>
<tr>
<td>3. Sandy gravel and/or gravel (GW and GP)</td>
<td>3,000</td>
<td>200</td>
<td>0.35</td>
</tr>
<tr>
<td>4. Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SP, SM, SC, GM and GC)</td>
<td>2,000</td>
<td>150</td>
<td>0.25</td>
</tr>
<tr>
<td>5. Clay, sandy clay, silty clay, clayey silt, silt and sandy silt (CL, ML, MH and CH)</td>
<td>1,500&lt;sup&gt;c&lt;/sup&gt;</td>
<td>100</td>
<td>–</td>
</tr>
</tbody>
</table>

For SI: 1 pound per square foot = 0.0479 kPa, 1 pound per square foot per foot = 0.157 kPa/m.

<sup>a</sup> Coefficient to be multiplied by the dead load.

<sup>b</sup> Lateral sliding resistance value to be multiplied by the contact area, as limited by Section 1804.3.

<sup>c</sup> Where the building official determines that in-place soils with an allowable bearing capacity of less than 1,500 psf are likely to be present at the site, the allowable bearing capacity shall be determined by a soils investigation.

<sup>d</sup> An increase of one-third is permitted when considering load combinations, including wind or earthquake loads, as permitted by Section 1605.3.2.

### 1805.2 Depth of footings. The minimum depth of footings below the undisturbed ground surface shall be 12 inches (305 mm). Where applicable, the depth of footings shall also conform to Sections 1805.2.1 through 1805.2.3.

#### 1805.2.1 Frost protection. Except where erected on solid rock or otherwise protected from frost, foundation walls, piers and other permanent supports of buildings and structures larger than 400 square feet (37 m²) in area or 10 feet (3048 mm) in height shall extend below the frost line of the locality, and spread footings of adequate size shall be provided where necessary to properly distribute the load within the allowable load-bearing value of the soil. Alternatively, such structures shall be supported on piles where solid earth or rock is not available. Footings shall not bear on frozen soils unless such frozen condition is of a permanent character.

#### 1805.2.2 Isolated footings. Footings on granular soil shall be so located that the line drawn between the lower edges of adjoining footings shall not have a slope steeper than 30 degrees (0.52 rad) with the horizontal, unless the material supporting the higher footing is braced or retained or otherwise laterally supported in an approved manner or a greater slope has been properly established by engineering analysis.

#### 1805.2.3 Shifting or moving soils. Where it is known that the top of subsoils are of a shifting or moving character, footings shall be carried to a sufficient depth to ensure stability.

### 1805.3 Footings on or adjacent to slopes. The placement of buildings and structures on or adjacent to slopes steeper than one unit vertical in three units horizontal (33.3-percent slope) shall conform to Sections 1805.3.1 through 1805.3.5.

#### 1805.3.1 Building clearance from ascending slopes. In general, buildings below slopes shall be set a sufficient distance from the slope to provide protection from slope drainage, erosion and shallow failures. Except as provided for in Section 1805.3.5 and Figure 1805.3.1, the following criteria will be assumed to provide this protection. Where the existing slope is steeper than one unit vertical in one unit horizontal (100-percent slope), the toe of the slope shall be assumed to be at the intersection of a horizontal plane drawn from the top of the foundation and a plane drawn tangent to the slope at an angle of 45 degrees (0.79 rad) to the horizontal. Where a retaining wall is constructed at the toe of the slope, the height of the slope shall be measured from the top of the wall to the top of the slope.

#### 1805.3.2 Footing setback from descending slope surface. Footings on or adjacent to slope surfaces shall be founded in firm material with an embedment and setback from the slope surface sufficient to provide vertical and lateral support for the footing without detrimental settlement. Except as provided for in Section 1805.3.5 and Figure 1805.3.1, the following setback is deemed adequate to meet the criteria. Where the slope is steeper than 1 unit vertical in 1 unit horizontal (100-percent slope), the required setback shall be measured from an imaginary
plane 45 degrees (0.79 rad) to the horizontal, projected upward from the toe of the slope.

1805.3.3 Pools. The setback between pools regulated by this code and slopes shall be equal to one half the building footing setback distance required by this section. That portion of the pool wall within a horizontal distance of 7 feet (2134 mm) from the top of the slope shall be capable of supporting the water in the pool without soil support.

1805.3.4 Foundation elevation. On graded sites, the top of any exterior foundation shall extend above the elevation of the street gutter at point of discharge or the inlet of an approved drainage device a minimum of 12 inches (305 mm) plus 2 percent. Alternate elevations are permitted subject to the approval of the building official, provided it can be demonstrated that required drainage to the point of discharge and away from the structure is provided at all locations on the site.

1805.3.5 Alternate setback and clearance. Alternate setbacks and clearances are permitted, subject to the approval of the building official. The building official is permitted to require an investigation and recommendation of a registered design professional to demonstrate that the intent of this section has been satisfied. Such an investigation shall include consideration of material, height of slope, slope gradient, load intensity and erosion characteristics of slope material.

1805.4 Footings. Footings shall be designed and constructed in accordance with Sections 1805.4.1 through 1805.4.6.

1805.4.1 Design. Footings shall be so designed that the allowable bearing capacity of the soil is not exceeded, and that differential settlement is minimized. The minimum width of footings shall be 12 inches (305 mm).

Footings in areas with expansive soils shall be designed in accordance with the provisions of Section 1805.8.

1805.4.1.1 Design loads. Footings shall be designed for the most unfavorable effects due to the combinations of loads specified in Section 1605.3. The dead load shall include the weight of foundations, footings and overlying fill. Reduced live loads, as specified in Section 1607.9 are permitted to be used in designing footings.

1805.4.1.2 Vibratory loads. Where machinery operations or other vibrations are transmitted through the foundation, consideration shall be given in the footing design to prevent detrimental disturbances of the soil.

1805.4.2 Concrete footings. The design, materials and construction of concrete footings shall comply with Sections 1805.4.2.1 through 1805.4.2.6 and the provisions of Chapter 19.

Exception: Where a specific design is not provided, concrete footings supporting walls of light-frame construction are permitted to be designed in accordance with Table 1805.4.2.

1805.4.2.1 Concrete strength. Concrete in footings shall have a specified compressive strength \( f'_{cu} \) of not less than 2,500 psi (17 237 kPa) at 28 days.

1805.4.2.2 Footing seismic ties. Where a structure is assigned to Seismic Design Category D, E or F in
accordance with Section 1616, individual spread footings founded on soil defined in Section 1615.1.1 as Site Class E or F shall be interconnected by ties. Ties shall be capable of carrying, in tension or compression, a force equal to the product of the larger footing load times the seismic coefficient $S_{DS}$ divided by 10 unless it is demonstrated that equivalent restraint is provided by reinforced concrete beams within slabs on grade or reinforced concrete slabs on grade.

1805.4.2.3 Plain concrete footings. In plain concrete footings, the edge thickness shall not be less than 8 inches (203 mm) for footings on soil.

**Exception:** For occupancies of Group R-3 and buildings less than two stories in height of light-frame construction, the required edge thickness is permitted to be 6 inches (152 mm), provided that the footing does not extend beyond a distance greater than the thickness of the footing on either side of the supported wall.

1805.4.2.4 Placement of concrete. Concrete footings shall not be placed through water unless a tremie or other method approved by the building official is used. Where placed under or in the presence of water, the concrete shall be deposited by approved means to ensure minimum segregation of the mix and negligible turbulence of the water.

1805.4.2.5 Protection of concrete. Concrete footings shall be protected from freezing during depositing and for a period of not less than 5 days thereafter. Water shall not be allowed to flow through the deposited concrete.

1805.4.2.6 Forming of concrete. Concrete footings are permitted to be cast against the earth where, in the opinion of the building official, soil conditions do not require forming. Where forming is required, forming shall be in accordance with Chapter 6 of ACI 318.

1805.4.3 Masonry-unit footings. The design, materials and construction of masonry-unit footings shall comply with Sections 1805.4.3.1 and 1805.4.3.2, and the provisions of Chapter 21.

**Exception:** Where a specific design is not provided, masonry-unit footings supporting walls of light-frame construction are permitted to be designed in accordance with Table 1805.4.2.

1805.4.3.1 Dimensions. Masonry-unit footings shall be laid in Type M or S mortar complying with Section 2103.7 and the depth shall not be less than twice the projection beyond the wall, pier or column. The width shall not be less than 8 inches (203 mm) wider than the wall supported thereon.

1805.4.3.2 Offsets. The maximum offset of each course in brick foundation walls stepped up from the footings shall be $1\frac{1}{2}$ inches (38 mm) where laid in single courses, and 3 inches (76 mm) where laid in double courses.

1805.4.4 Steel grillage footings. Grillage footings of structural steel shapes shall be separated with approved steel spacers and shall be entirely encased in concrete with at least 6 inches (152 mm) on the bottom and at least 4 inches (102 mm) at all other points. The spaces between the shapes shall be completely filled with concrete or cement grout.

1805.4.5 Timber footings. Timber footings are permitted for buildings of Type V construction and as otherwise approved by the building official. Such footings shall be treated in accordance with AWPA C2 or C3. Treated timbers are not required where placed entirely below permanent water level, or where used as capping for wood piles that project above the water level over submerged or marsh lands. The compressive stresses perpendicular to grain in untreated timber footings supported upon piles shall not exceed 70 percent of the allowable stresses for the species and grade of timber as specified in the AFPA NDS.

1805.4.6 Wood foundations. Wood foundation systems shall be designed and installed in accordance with AWPA Technical Report No. 7. Lumber and plywood shall be treated in accordance with AWPA C22 and shall be identified in accordance with Section 2303.1.8.1.

1805.5 Foundation walls. Concrete and masonry foundation walls shall be designed in accordance with Chapter 19 or 21. Foundation walls that are laterally supported at the top and bottom and within the parameters of Tables 1805.5(1) through 1805.5(4) are permitted to be designed and constructed in accordance with Sections 1805.5.1 through 1805.5.4.

1805.5.1 Foundation wall thickness. The minimum thickness of concrete and masonry foundation walls shall comply with Sections 1805.5.1.1 through 1805.5.1.3.

1805.5.1.1 Thickness based on walls supported. The thickness of foundation walls shall not be less than the thickness of the wall supported, except that foundation walls of at least 8-inch (203 mm) nominal width are permitted to support brick-veneered frame walls and 10-inch-wide (254 mm) cavity walls provided the requirements of Section 1805.5.1.2 are met. Corbelling of masonry shall be in accordance with
Section 2104.2. Where an 8-inch (203 mm) wall is corbeled, the top corbel shall be a full course of headers at least 6 inches (152 mm) in length, extending not higher than the bottom of the floor framing.

1805.5.1.2 Thickness based on soil loads, unbalanced backfill height and wall height. The thickness of foundation walls shall comply with the requirements of Table 1805.5(1) for plain masonry and plain concrete walls or Table 1805.5(2), 1805.5(3) or 1805.5(4) for reinforced concrete and masonry walls. When using the tables, masonry shall be laid in running bond and the mortar shall be Type M or S.

Unbalanced backfill height is the difference in height of the exterior and interior finish ground levels. Where an interior concrete slab is provided, the unbalanced backfill height shall be measured from the exterior finish ground level to the top of the interior concrete slab.

1805.5.1.3 Rubble stone. Foundation walls of rough or random rubble stone shall not be less than 16 inches (406 mm) thick. Rubble stone shall not be used for foundations for structures in Seismic Design Category C, D, E or F.

1805.5.2 Foundation wall materials. Foundation walls constructed in accordance with Tables 1805.5(1), 1805.5(2), 1805.5(3) or 1805.5(4) shall comply with the following:

1. Vertical reinforcement shall have a minimum yield strength of 60,000 psi (414 MPa).
2. The specified location of the reinforcement shall equal or exceed the effective depth distance, \( d \), noted in Tables 1805.5(2), 1805.5(3) and 1805.5(4) and shall be measured from the face of the soil side of the wall to the center of vertical reinforcement. The reinforcement shall be placed within the tolerances specified in ACI 530.1/ASCE 6/TMS 402, Article 3.4E1 of the specified location.
3. Concrete shall have a specified compressive strength of not less than 2,500 psi (17.2 MPa) at 28 days.
4. Grout shall have a specified compressive strength of not less than 2,000 psi (13.8 MPa) at 28 days.
5. Hollow masonry units shall comply with ASTM C 90 and shall be installed with Type M or S mortar.

1805.5.3 Alternative foundation wall reinforcement. In lieu of the reinforcement provisions in Table 1805.5(2), 1805.5(3) or 1805.5(4), alternative reinforcing bar sizes and spacings having an equivalent cross-sectional area of reinforcement per linear foot (mm) of wall is permitted to be used, provided the spacing of reinforcement does not exceed 72 inches (1829 mm) and reinforcing bar sizes do not exceed No. 11.

1805.5.4 Hollow masonry walls. At least 4 inches (102 mm) of solid masonry shall be provided at girder supports at the top of hollow masonry unit foundation walls.

1805.5.5 Foundation wall drainage. Foundation walls shall be designed to support the weight of the full hydrostatic pressure of undrained backfill unless a drainage system...
is installed in accordance with Sections 1806.4.2 and 1806.4.3.

**1805.5.6 Pier and curtain wall foundations.** Except in Seismic Design Categories D, E and F, pier and curtain wall foundations are permitted to be used to support light-frame construction not more than two stories in height, provided the following requirements are met:

1. All load-bearing walls shall be placed on continuous concrete footings bonded integrally with the exterior wall footings.
2. The minimum actual thickness of a load-bearing masonry wall shall not be less than 4 inches (102 mm) nominal or 35/8 inches (92 mm) actual thickness, and shall be bonded integrally with piers spaced 6 feet (1829 mm) on center.
3. Piers shall be constructed in accordance with Chapter 21 and the following:
   3.1. The unsupported height of the masonry piers shall not exceed 10 times their least dimension.
   3.2. Where structural clay tile or hollow concrete masonry units are used for piers supporting beams and girders, the cellular spaces shall be filled solidly with concrete or Type M or S mortar.
   Exception: Unfilled hollow piers are permitted where the unsupported height of the pier is not more than four times its least dimension.
3.3. Hollow piers shall be capped with 4 inches (102 mm) of solid masonry or concrete or the cavities of the top course shall be filled with concrete or grout.
4. The maximum height of a 4-inch (102 mm) load-bearing masonry foundation wall supporting wood-framed walls and floors shall not be more than 4 feet (1219 mm) in height.
5. The unbalanced fill for 4-inch (102 mm) foundations walls shall not exceed 24 inches (610 mm) for solid masonry, nor 12 inches (305 mm) for hollow masonry.

**1805.6 Foundation plate or sill bolting.** Wood foundation plates or sills shall be bolted or strapped to the foundation or foundation wall as provided in Chapter 23.

**1805.7 Designs employing lateral bearing.** Designs to resist both axial and lateral loads employing posts or poles as columns embedded in earth or embedded in concrete footings in the earth shall conform to the requirements of Sections 1805.7.1 through 1805.7.3.

**1805.7.1 Limitations.** The design procedures outlined in this section are subject to the following limitations:

1. The frictional resistance for structural walls and slabs on silts and clays shall be limited to one-half of the normal force imposed on the soil by the weight of the footing or slab.
2. Posts embedded in earth shall not be used to provide lateral support for structural or nonstructural materials such as plaster, masonry or concrete unless bracing is provided that develops the limited deflection required.

Wood poles shall be treated in accordance with AWPA C2 or C4.

**1805.7.2 Design criteria.** The depth to resist lateral loads shall be determined by the design criteria established in Sections 1805.7.2.1 through 1805.7.2.3, or by other methods approved by the building official.

**1805.7.2.1 Nonconstrained.** The following formula shall be used in determining the depth of embedment required to resist lateral loads where no constraint is provided at the ground surface, such as rigid floor or rigid ground surface pavement, and where no lateral constraint is provided above the ground surface, such as a structural diaphragm.

\[
d = 0.5A[1 + (1 + (4.36h/A))^{1/2}]
\]

(Equation 18-1)

where:

- \( A = 2.34P/S_1/b \)
- \( b = \) diameter of round post or footing or diagonal dimension of square post or footing, feet (m).
- \( d = \) depth of embedment in earth in feet (m) but not over 12 feet (3658 mm) for purpose of computing lateral pressure.
- \( h = \) distance in feet (m) from ground surface to point of application of “P.”
- \( P = \) applied lateral force in pounds (kN).
- \( S_1 = \) allowable lateral soil-bearing pressure as set forth in Section 1804.3 based on a depth of one-third the depth of embedment in pounds per square foot (kPa).
- \( S_3 = \) allowable lateral soil-bearing pressure as set forth in Section 1804.3 based on a depth equal to the depth of embedment in pounds per square foot (kPa).
### Table 1805.5(1)

**Plain Masonry and Plain Concrete Foundation Walls**

#### Plain Masonry

<table>
<thead>
<tr>
<th>Wall Height (feet)</th>
<th>Height of Unbalanced Backfill (feet)</th>
<th>Minimum Nominal Wall Thickness (inches)</th>
<th>Soil Classes and Lateral Soil Load $a$ (psf per foot below natural grade)</th>
<th>SC, MH, ML-CL and Inorganic CL Soils</th>
</tr>
</thead>
<tbody>
<tr>
<td>7</td>
<td>4 (or less)</td>
<td>8</td>
<td>8</td>
<td>10 (solid$e$)</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>8</td>
<td>10</td>
<td>8 (solid$e$)</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>10</td>
<td>12</td>
<td>Note $d$</td>
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<tr>
<td></td>
<td>7</td>
<td>12</td>
<td>12 (solid$e$)</td>
<td>Note $d$</td>
</tr>
<tr>
<td>8</td>
<td>4 (or less)</td>
<td>8</td>
<td>8</td>
<td>Note $d$</td>
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<td>10</td>
<td>8 (solid$e$)</td>
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<td>12 (solid$e$)</td>
<td>Note $d$</td>
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<td>7</td>
<td>12 (solid$e$)</td>
<td>12 (solid$e$)</td>
<td>Note $d$</td>
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<td>8</td>
<td>12 (solid$e$)</td>
<td>Note $d$</td>
<td>Note $d$</td>
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<td>9</td>
<td>4 (or less)</td>
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<td>Note $d$</td>
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<td>8 (solid$e$)</td>
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<td>12</td>
<td>12 (solid$e$)</td>
<td>Note $d$</td>
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<td>12 (solid$e$)</td>
<td>12 (solid$e$)</td>
<td>Note $d$</td>
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<td></td>
<td>8</td>
<td>12 (solid$e$)</td>
<td>Note $d$</td>
<td>Note $d$</td>
</tr>
</tbody>
</table>

#### Plain Concrete

<table>
<thead>
<tr>
<th>Wall Height (feet)</th>
<th>Height of Unbalanced Backfill (feet)</th>
<th>Minimum Nominal Wall Thickness (inches)</th>
<th>Soil Classes and Lateral Soil Load $a$ (psf per foot below natural grade)</th>
<th>SC, MH, ML-CL and Inorganic CL Soils</th>
</tr>
</thead>
<tbody>
<tr>
<td>7</td>
<td>4 (or less)</td>
<td>7½</td>
<td>7½</td>
<td>7½</td>
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<tr>
<td></td>
<td>5</td>
<td>7½</td>
<td>7½</td>
<td>7½</td>
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<td></td>
<td>6</td>
<td>7½</td>
<td>7½</td>
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<td>7½</td>
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<tr>
<td>8</td>
<td>4 (or less)</td>
<td>7½</td>
<td>7½</td>
<td>7½</td>
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<td></td>
<td>5</td>
<td>7½</td>
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<td>6</td>
<td>7½</td>
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<td>8</td>
<td>7½</td>
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<tr>
<td></td>
<td>9</td>
<td>7½</td>
<td>7½</td>
<td>7½</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot per foot = 0.157 kPa/m.

- **a.** For design lateral soil loads, see Section 1610. Soil classes are in accordance with the Unified Soil Classification System and design lateral soil loads are for moist soil conditions without hydrostatic pressure.
- **b.** Provisions for this table are based on construction requirements specified in Section 1805.5.2.
- **c.** Solid grouted hollow units or solid masonry units.
- **d.** A design in compliance with Chapter 21 or reinforcement in accordance with Table 1805.5(2) is required.
- **e.** A design in compliance with Chapter 19 is required.
**TABLE 1805.5(2)**

8-INCH REINFORCED CONCRETE AND MASONRY FOUNDATION WALLS WHERE \( d \geq 5 \) INCHES\(^a, b, c\)

<table>
<thead>
<tr>
<th>WALL HEIGHT (feet)</th>
<th>HEIGHT OF UNBALANCED BACKFILL (feet)</th>
<th>VERTICAL REINFORCEMENT</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>30</td>
</tr>
<tr>
<td>7</td>
<td>4 (or less)</td>
<td>#4 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#5 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#5 at 40&quot; o.c.</td>
</tr>
<tr>
<td>8</td>
<td>4 (or less)</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#6 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>8</td>
<td>#6 at 40&quot; o.c.</td>
</tr>
<tr>
<td>9</td>
<td>4 (or less)</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#5 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>8</td>
<td>#5 at 40&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>9</td>
<td>#6 at 40&quot; o.c.</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot per foot = 0.157 kPa/m.

\(^a\) For design lateral soil loads, see Section 1610. Soil classes are in accordance with the Unified Soil Classification System and design lateral soil loads are for moist soil conditions without hydrostatic pressure.

\(^b\) Provisions for this table are based on construction requirements specified in Section 1805.5.2.

\(^c\) For alternative reinforcement, see Section 1805.5.3.

---

**TABLE 1805.5(3)**

10-INCH REINFORCED CONCRETE AND MASONRY FOUNDATION WALLS WHERE \( d \geq 6.75 \) INCHES\(^a, b, c\)

<table>
<thead>
<tr>
<th>WALL HEIGHT (feet)</th>
<th>HEIGHT OF UNBALANCED BACKFILL (feet)</th>
<th>VERTICAL REINFORCEMENT</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>30</td>
</tr>
<tr>
<td>7</td>
<td>4 (or less)</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#5 at 48&quot; o.c.</td>
</tr>
<tr>
<td>8</td>
<td>4 (or less)</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#5 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>8</td>
<td>#5 at 40&quot; o.c.</td>
</tr>
<tr>
<td>9</td>
<td>4 (or less)</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 56&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#5 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>8</td>
<td>#5 at 40&quot; o.c.</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot per foot = 0.157 kPa/m.

\(^a\) For design lateral soil loads, see Section 1610. Soil classes are in accordance with the Unified Soil Classification System and design lateral soil loads are for moist soil conditions without hydrostatic pressure.

\(^b\) Provisions for this table are based on construction requirements specified in Section 1805.5.2.

\(^c\) For alternative reinforcement, see Section 1805.5.3.
### TABLE 1805.5(4)

12-INCH REINFORCED CONCRETE AND MASONRY FOUNDATION WALLS WHERE $d \geq 8.75$ INCH$^{a, b, c}$

<table>
<thead>
<tr>
<th>WALL HEIGHT (feet)</th>
<th>HEIGHT OF UNBALANCED BACKFILL (feet)</th>
<th>VERTICAL REINFORCEMENT</th>
</tr>
</thead>
<tbody>
<tr>
<td>7</td>
<td>4 (or less)</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td>8</td>
<td>4 (or less)</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#4 at 64&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>8</td>
<td>#4 at 48&quot; o.c.</td>
</tr>
<tr>
<td>9</td>
<td>4 (or less)</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>5</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>6</td>
<td>#4 at 72&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>7</td>
<td>#4 at 64&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>8</td>
<td>#4 at 48&quot; o.c.</td>
</tr>
<tr>
<td></td>
<td>9</td>
<td>#5 at 56&quot; o.c.</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot per foot = 0.157 kPa/m.

a. For design lateral soil loads, see Section 1610. Soil classes are in accordance with the Unified Soil Classification System and design lateral soil loads are for moist soil conditions without hydrostatic pressure.

b. Provisions for this table are based on construction requirements specified in Section 1805.5.2.

c. For alternative reinforcement, see Section 1805.5.3.

#### 1805.7.2.2 Constrained

The following formula shall be used to determine the depth of embedment required to resist lateral loads where constraint is provided at the ground surface, such as a rigid floor or pavement.

$$d^2 = 4.25 \left( \frac{Ph}{S_3 b} \right) \quad \text{(Equation 18-2)}$$

or alternately

$$d^2 = 4.25 \left( \frac{M_b}{S_3 b} \right) \quad \text{(Equation 18-3)}$$

where:

$M_b$ = moment in the post at grade, in foot-pounds (kN-m).

#### 1805.7.2.3 Vertical load

The resistance to vertical loads shall be determined by the allowable soil-bearing pressure set forth in Table 1804.2.

#### 1805.7.3 Backfill

The backfill in the annular space around columns not embedded in poured footings shall be by one of the following methods:

1. Backfill shall be of concrete with an ultimate strength of 2,000 pounds per square inch (13.8 MPa) at 28 days. The hole shall not be less than 4 inches (102 mm) larger than the diameter of the column at its bottom or 4 inches (102 mm) larger than the diagonal dimension of a square or rectangular column.

2. Backfill shall be of clean sand. The sand shall be thoroughly compacted by tamping in layers not more than 8 inches (203 mm) in depth.

#### 1805.8 Design for expansive soils

Footings or foundations for buildings and structures founded on expansive soils shall be designed in accordance with Section 1805.8.1 or 1805.8.2.

Footings or foundation design need not comply with Section 1805.8.1 or 1805.8.2 where the soil is removed in accordance with Section 1805.8.3, nor where the building official approves stabilization of the soil in accordance with Section 1805.8.4.

#### 1805.8.1 Foundations

Footings or foundations placed on or within the active zone of expansive soils shall be designed to resist differential volume changes and to prevent structural damage to the supported structure.
Deflection and racking of the supported structure shall be limited to that which will not interfere with the usability and serviceability of the structure.

Foundations placed below where volume change occurs or below expansive soil shall comply with the following provisions:

1. Foundations extending into or penetrating expansive soils shall be designed to prevent uplift of the supported structure.
2. Foundations penetrating expansive soils shall be designed to resist forces exerted on the foundation due to soil volume changes or shall be isolated from the expansive soil.

1805.8.2 Slab-on-ground foundations. Slab-on-ground, mat or raft foundations on expansive soils shall be designed and constructed in accordance with WRI/CRSI Design of Slab-on-Ground Foundations or PTI Design and Construction of Post-Tensioned Slabs-On-Ground.

Exception: Slab-on-ground systems that have performed adequately in soil conditions similar to those encountered at the building site are permitted subject to the approval of the building official.

1805.8.3 Removal of expansive soil. Where expansive soil is removed in lieu of designing footings or foundations in accordance with Section 1805.8.1 or 1805.8.2, the soil shall be removed to a depth sufficient to ensure a constant moisture content in the remaining soil. Fill material shall not contain expansive soils and shall comply with Section 1803.4.

Exception: Expansive soil need not be removed to the depth of constant moisture, provided the confining pressure in the expansive soil created by the fill and supported structure exceeds the swell pressure.

1805.8.4 Stabilization. Where the active zone of expansive soils is stabilized in lieu of designing footings or foundations in accordance with Section 1805.8.1 or 1805.8.2, the soil shall be stabilized by chemical, dewatering, presaturation or equivalent techniques.

1805.9 Seismic requirements. See Section 1910 for additional requirements for footings and foundations of structures assigned to Seismic Design Categories C, D, E and F.

For structures assigned to Seismic Design Categories D, E and F, provisions of ACI 318, Sections 21.8.1 to 21.8.3, shall apply when not in conflict with the provisions of Section 1805. Concrete shall have a specified compressive strength of not less than 3,000 psi (20.68 MPa) at 28 days.

Exception: Group R or Group U Occupancies of light-frame construction and two stories or less in height are permitted to use concrete with a specified compressive strength of not less than 2,500 psi (17.2 MPa) at 28 days.

SECTION 1806
DAMPPROOFING AND WATERPROOFING

1806.1 Where required. Walls or portions thereof that retain earth and enclose interior spaces and floors below grade shall be waterproofed and dampproofed in accordance with this section, with the exception of those spaces containing groups other than residential and institutional where such omission is not detrimental to the building or occupancy.

Ventilation for crawl spaces shall comply with Section 1202.4.

1806.1.1 Story above grade. Where a basement is considered a story above grade and the finished ground level adjacent to the basement wall is below the basement floor elevation for 25 percent or more of the perimeter, the floor and walls shall be dampproofed in accordance with Section 1806.2 and a foundation drain shall be installed in accordance with Section 1806.4.2. The foundation drain shall be installed around the portion of the perimeter where the basement floor is below ground level. The provisions of Sections 1802.2.3, 1806.3 and 1806.4.1 shall not apply in this case.

1806.1.2 Underfloor space. The finished ground level of an underfloor space such as a crawl space shall not be located below the bottom of the footings. Where there is evidence that the ground water table rises to within 6 inches (152 mm) of the ground level at the outside building perimeter or where there is evidence that the surface water does not readily drain from the building site, the ground level of the underfloor space shall be as high as the outside finished ground level, unless an approved drainage system is provided. The provisions of Sections 1802.2.3, 1806.2, 1806.3 and 1806.4 shall not apply in this case.

1806.1.2.1 Flood hazard areas. For buildings and structures in flood hazard areas as established in Section 1612.3, the finished ground level of an underfloor space such as a crawl space shall be equal to or higher than the outside finished ground level.

1806.1.3 Ground-water control. Where the ground-water table is lowered and maintained at an elevation not less than 6 inches (152 mm) below the bottom of the lowest floor, the floor and walls shall be dampproofed in accordance with Section 1806.2. The design of the system to lower the ground-water table shall be based on accepted principles of engineering that shall consider, but not necessarily be limited to, permeability of the soil, rate
at which water enters the drainage system, rated capacity of pumps, head against which pumps are to pump; and the rated capacity of the disposal area of the system.

1806.2 Dampproofing required. Where hydrostatic pressure will not occur as determined by Section 1802.2.3, floors and walls for other than wood foundation systems shall be dampproofed in accordance with this section. Wood foundation systems shall be constructed in accordance with AFPA TR7.

1806.2.1 Floors. Dampproofing materials for floors shall be installed between the floor and the base course required by Section 1806.4.1, except where a separate floor is provided above a concrete slab.

Where installed beneath the slab, dampproofing shall consist of not less than 6-mil (0.006 inch; 0.152 mm) polyethylene with joints lapped not less than 6 inches (152 mm), or other approved methods or materials. Where permitted to be installed on top of the slab, dampproofing shall consist of mopped-on bitumen, not less than 4-mil (0.004 inch; 0.102 mm) polyethylene, or other approved methods or materials. Joints in the membrane shall be lapped and sealed in accordance with the manufacturer’s installation instructions.

1806.2.2 Walls. Dampproofing materials for walls shall be installed on the exterior surface of the wall, and shall extend from the top of the footing to above ground level.

Dampproofing shall consist of a bituminous material, 3 pounds per square yard (16 N/m²) of acrylic modified cement, 3/8-inch (3.2 mm) coat of surface-bonding mortar complying with ASTM C 887, any of the materials permitted for waterproofing by Section 1806.3.2, or other approved methods or materials.

1806.2.2.1 Surface preparation of walls. Prior to application of dampproofing materials on concrete walls, holes and recesses resulting from the removal of form ties shall be sealed with a bituminous material or other approved methods or materials. Unit masonry walls shall be parged on the exterior surface below ground level with not less than 3/8 inch (9.5 mm) of portland cement mortar. The parging shall be coved at the footing.

Exception: Parging of unit masonry walls is not required where a material is approved for direct application to the masonry.

1806.3 Waterproofing required. Where the ground-water investigation required by Section 1802.2.3 indicates that a hydrostatic pressure condition exists, and the design does not include a ground-water control system as described in Section 1806.1.3, walls and floors shall be waterproofed in accordance with this section.

1806.3.1 Floors. Floors required to be waterproofed shall be of concrete, designed and constructed to withstand the hydrostatic pressures to which the floors will be subjected.

Waterproofing shall be accomplished by placing a membrane of rubberized asphalt, butyl rubber, or not less than 60-mil (0.060 inch; 0.152 mm) polyvinyl chloride with joints lapped not less than 6 inches (152 mm) or other approved materials under the slab. Joints in the membrane shall be lapped and sealed in accordance with the manufacturer’s installation instructions.

1806.3.2 Walls. Walls required to be waterproofed shall be of concrete or masonry and shall be designed and constructed to withstand the hydrostatic pressures and other lateral loads to which the walls will be subjected.

Waterproofing shall be applied from the bottom of the wall to not less than 12 inches (305 mm) above the maximum elevation of the ground water table. The remainder of the wall shall be dampproofed in accordance with Section 1806.2.2. Waterproofing shall consist of two-ply hot-mopped felts, not less than 6-mil (0.006 inch; 0.152 mm) polyvinyl chloride, 40-mil (0.040 inch; 1.02 mm) polymer-modified asphalt, 6-mil (0.006 inch; 0.152 mm) polyethylene or other approved methods or materials capable of bridging nonstructural cracks. Joints in the membrane shall be lapped and sealed in accordance with the manufacturer’s installation instructions.

1806.3.2.1 Surface preparation of walls. Prior to the application of waterproofing materials on concrete or masonry walls, the walls shall be prepared in accordance with Section 1806.2.2.1.

1806.3.3 Joints and penetrations. Joints in walls and floors, joints between the wall and floor, and penetrations of the wall and floor shall be made watertight utilizing approved methods and materials.

1806.4 Subsoil drainage system. Where a hydrostatic pressure condition does not exist, dampproofing shall be provided and a base shall be installed under the floor and a drain installed around the foundation perimeter. A subsoil drainage system designed and constructed in accordance with Section 1806.1.3 shall be deemed adequate for lowering the ground-water table.

1806.4.1 Floor base course. Floors of basements, except as provided for in Section 1806.1.1, shall be placed over a floor base course not less than 4 inches (102 mm) in thickness that consists of gravel or crushed stone containing not more than 10 percent of material that passes through a No. 4 (4.75 mm) sieve.

Exception: Where a site is located in well-drained gravel or sand/gravel mixture soils, a floor base course is not required.
1806.4.2 Foundation drain. A drain shall be placed around the perimeter of a foundation that consists of gravel or crushed stone containing not more than 10 percent material that passes through a No. 4 (4.75 mm) sieve. The drain shall extend a minimum of 12 inches (305 mm) beyond the outside edge of the footing. The thickness shall be such that the bottom of the drain is not higher than the bottom of the base under the floor, and that the top of the drain is not less than 6 inches (152 mm) above the top of the footing. The top of the drain shall be covered with an approved filter membrane material. Where a drain tile or perforated pipe is used, the invert of the pipe or tile shall not be higher than the floor elevation. The top of joints or the top of perforations shall be protected with an approved filter membrane material. The pipe or tile shall be placed on not less than 2 inches (51 mm) of gravel or crushed stone complying with Section 1806.4.1, and shall be covered with not less than 6 inches (152 mm) of the same material.

1806.4.3 Drainage discharge. The floor base and foundation perimeter drain shall discharge by gravity or mechanical means into an approved drainage system that complies with the International Plumbing Code.

Exception: Where a site is located in well-drained gravel or sand/gravel mixture soils, a dedicated drainage system is not required.

SECTION 1807
PIER AND PILE FOUNDATIONS

1807.1 Definitions. The following words and terms shall, for the purposes of this section, have the meanings shown herein.

PIER FOUNDATIONS. Pier foundations consist of isolated masonry or cast-in-place concrete structural elements extending into firm materials. Piers are relatively short in comparison to their width, with lengths less than or equal to 12 times the least horizontal dimension of the pier. Piers derive their load-carrying capacity through skin friction, through end bearing, or a combination of both.

Belled piers. Bellied piers are cast-in-place concrete piers constructed with a base that is larger than the diameter of the remainder of the pier. The bellied base is designed to increase the load-bearing area of the pier in end bearing.

PILE FOUNDATIONS. Pile foundations consist of concrete, wood or steel structural elements either driven into the ground or cast-in-place. Piles are relatively slender in comparison to their length, with lengths exceeding 12 times the least horizontal dimension. Piles derive their load-carrying capacity through skin friction, through end bearing, or a combination of both.

Augered uncased piles. Augered uncased piles are constructed by depositing concrete into an uncased augered hole, either during or after the withdrawal of the auger.

Caisson piles. Caisson piles are cast-in-place concrete piles extending into bedrock. The upper portion of a caisson pile consists of a cased pile that extends to the bedrock. The lower portion of the caisson pile consists of an uncased socket drilled into the bedrock.

Concrete-filled steel pipe and tube piles. Concrete-filled steel pipe and tube piles are constructed by driving a steel pipe or tube section into the soil and filling the pipe or tube section with concrete. The steel pipe or tube section is left in place during and after the deposition of the concrete.

Driven uncased piles. Driven uncased piles are constructed by driving a steel shell into the soil to shore an unexcavated hole that is later filled with concrete. The steel casing is lifted out of the hole during the deposition of the concrete.

Enlarged base piles. Enlarged base piles are cast-in-place concrete piles constructed with a base that is larger than the diameter of the remainder of the pile. The enlarged base is designed to increase the load-bearing area of the pile in end bearing.

1807.2 Piers and piles—general requirements.

1807.2.1 General. Pier and pile foundations shall be designed and installed on the basis of a foundation investigation as defined in Section 1802, unless sufficient data upon which to base the design and installation is available.

The investigation and report provisions of Section 1802 shall be expanded to include, but not be limited to, the following:

1. Recommended pier or pile types and installed capacities.
2. Driving criteria.
3. Installation procedures.
4. Field inspection and reporting procedures (to include procedures for verification of the installed bearing capacity where required).
5. Pier or pile load test requirements.
6. Durability of pier or pile materials.
7. Designation of bearing stratum or strata.
8. Reductions for group action, where necessary.

1807.2.2 Special types of piles. The use of types of piles not specifically mentioned herein is permitted, subject to
the approval of the building official, upon the submission
of acceptable test data, calculations and other information
relating to the structural properties and load capacity of
such piles. The allowable stresses shall not in any case
exceed the limitations specified herein.

1807.2.3 Pile caps. Pile caps shall be of reinforced con-
crete. The soil immediately below the pile cap shall not
be considered as carrying any vertical load. The tops
of piles shall be embedded not less than 3 inches (76 mm)
into pile caps and the caps shall extend at least 4 inches
(102 mm) beyond the edges of piles. The tops of piles
shall be cut back to sound material before capping.

1807.2.4 Stability. Piers or piles shall be braced to pro-
vide lateral stability in all directions. Three or more piles
connected by a rigid cap shall be considered braced, pro-
vided that the piles are located in radial directions from
the centroid of the group not less than 60 degrees (1 rad)
apart. A two-pile group in a rigid cap shall be considered
to be braced along the axis connecting the two piles.
Methods used to brace piers or piles shall be subject to the
approval of the building official.

Piles supporting walls shall be driven alternately in
lines spaced at least 1 foot (305 mm) apart and located
symmetrically under the center of gravity of the wall load
carried, unless effective measures are taken to provide for
eccentricity and lateral forces, or the wall piles are ade-
quately braced to provide for lateral stability. A single
row of piles without lateral bracing is permitted for one-
and two-family dwellings and lightweight construction
not exceeding two stories or 35 feet (10 668 mm) in
height, provided the centers of the piles are located with-
in the width of the foundation wall.

1807.2.5 Structural integrity. Piers or piles shall be
installed in such a manner and sequence as to prevent dis-
tortion or damage to piles being installed or already in
place to the extent that such distortion or damage affects
the structural integrity of the piles.

1807.2.6 Spacing. The center-to-center spacing of piers
or piles shall be as recommended in the soils report.

1807.2.7 Splices. Splices shall be constructed so as to
provide and maintain true alignment and position of the
component parts of the pier or pile during installation
and subsequent thereto and shall be of adequate strength to
transmit the vertical and lateral loads and moments occur-
ring at the location of the splice during driving and under
service loading. Splices shall develop not less than 50
percent of the least capacity of the pier or pile in bending.
In addition, splices occurring in the upper 10 feet (3048
mm) of the embedded portion of the pier or pile shall be
capable of resisting at allowable working stresses the
moment and shear that would result from an assumed
eccentricity of the pier or pile load of 3 inches (76 mm),
or the pier or pile shall be braced in accordance with
Section 1807.2.4 to other piers or piles that do not have
splices in the upper 10 feet (3048 mm) of embedment.

1807.2.8 Allowable pier or pile loads.

1807.2.8.1 Determination of allowable loads. The
allowable axial and lateral loads on piers or piles shall
be determined by an approved formula, load tests or
method of analysis.

1807.2.8.2 Driving criteria. The allowable compres-
sive load on any pile where determined by the appli-
cation of an approved driving formula shall not
exceed 40 tons (356 kN). For allowable loads above
40 tons (356 kN), the wave equation method of analy-
sis shall be used to estimate pile driveability of both
driving stresses and net displacement per blow at the
ultimate load. Allowable loads shall be verified by
load tests in accordance with Section 1807.2.8.3. The
formula or wave equation load shall be determined for
gravity-drop or power-actuated hammers and the
hammer energy used shall be the maximum consistent
with the size, strength and weight of the driven piles.
The use of a follower is permitted only with the
approval of the building official. The introduction of
fresh hammer cushion or pile cushion material just
prior to final penetration is not permitted.

1807.2.8.3 Load tests. Where greater compressive
loads per pier or pile than permitted by Section
1807.2.10 are desired or where the design load for any
pier or pile foundation is in doubt, control test piers
or piles shall be tested in accordance with ASTM D 1143
or ASTM D 4945. At least one pier or pile shall be test
loaded in each area of uniform subsoil conditions.
Where required by the building official, additional
piers or piles shall be load tested where necessary to
establish the safe design capacity. The resulting
allowable loads shall not be more than one-half of that
test load, which produces a permanent net settlement
of not more than 0.01 inch per ton (0.0285 mm/kN) of
test load, and not more than 3/4 inch (19.1 mm). In
subsequent driving of the balance of foundation piles,
all piles shall be deemed to have a supporting capaci-
ty equal to the control pile where such piles are of the
same type, size and relative length as the test pile; are
installed using the same or comparable methods and
equipment as the test pile; are installed in similar sub-
soil conditions as the test pile; and where the rate of
penetration (e.g., net displacement per blow) of such
piles is equal to or less than that of the test pile
through a comparable driving distance.
**1807.2.8.4 Allowable frictional resistance.** The assumed frictional resistance developed by any pier or uncased cast-in-place pile shall not exceed one-sixth of the bearing value of the soil material at minimum depth as set forth in Table 1804.2, up to a maximum of 500 pounds per square foot (24 kPa), unless a greater value is allowed by the building official after a soil investigation as specified in Section 1802 is submitted. Frictional resistance and bearing resistance shall not be assumed to act simultaneously unless recommended by a soil investigation as specified in Section 1802.

**1807.2.8.5 Uplift capacity.** Where required by the design, the uplift capacity of a single pier or pile shall be determined in accordance with ASTM D 3689, or an approved method of analysis based on a minimum factor of safety of three. The maximum allowable uplift load shall be one-half that load which produces an upward movement of the pier or pile but equal to the gross elastic extension of the pier or pile plus 0.1 inch (2.5 mm). For pile groups subjected to uplift, the allowable working uplift load for the group shall be the lesser of:

1. The proposed individual pile uplift working load times the number of piles in the group.
2. Two-thirds of the effective weight of the pile group and the soil contained within a block defined by the perimeter of the group and the length of the pile.

**1807.2.8.6 Load-bearing capacity.** Piers, individual piles and groups of piles shall develop ultimate load capacities of at least twice the design working loads in the designated load-bearing layers. Analysis shall show that no soil layer underlying the designated load-bearing layers causes the load-bearing capacity safety factor to be less than two.

**1807.2.8.7 Bent piers or piles.** The load-bearing capacity of piers or piles discovered to have a sharp or sweeping bend shall be determined by an approved method of analysis or by load testing a representative pier or pile.

**1807.2.8.8 Overloads on piers or piles.** The maximum compressive load on any pier or pile due to mislocation shall not exceed 110 percent of the allowable design load.

**1807.2.9 Lateral support.**

**1807.2.9.1 General.** Any soil other than fluid soil shall be deemed to afford sufficient lateral support to the pier or pile to prevent buckling and to permit the design of the pier or pile in accordance with accepted engineering practice and the applicable provisions of this code.

**1807.2.9.2 Unbraced piles.** Piles standing unbraced in air, water or in fluid soils shall be designed as columns in accordance with the provisions of this code. Such piles driven into firm ground can be considered fixed and laterally supported at 5 feet (1524 mm) below the ground surface and in soft material at 10 feet (3048 mm) below the ground surface unless otherwise prescribed by the building official after a foundation investigation by an approved agency.

**1807.2.9.3 Allowable lateral load.** Where required by the design, the lateral load capacity of a pier, a single pile or a pile group shall be determined by an approved method of analysis or by lateral load tests to at least twice the proposed design working load. The resulting allowable load shall not be more than one-half of that test load that produces a gross lateral movement of 1 inch (25 mm) at the ground surface.

**1807.2.10 Use of higher allowable pier or pile stresses.** Allowable stresses greater than those specified for piers or for each pile type in Sections 1808 and 1809 are permitted where supporting data justifying such higher stresses is filed with the building official. Such substantiating data shall include:

1. A soils investigation in accordance with Section 1802.
2. Pier or pile load tests in accordance with Section 1807.2.8.3, regardless of the load supported by the pier or pile.

The design and installation of the pier or pile foundation shall be under the direct supervision of a registered design professional knowledgeable in the field of soil mechanics and pier or pile foundations who shall certify to the building official that the piers or piles as installed satisfy the design criteria.

**1807.2.11 Piles in subsiding areas.** Where piles are driven through subsiding fills or other subsiding strata and derive support from underlying firmer materials, consideration shall be given to the downward frictional forces that may be imposed on the piles by the subsiding upper strata.

Where the influence of subsiding fills is considered as imposing loads on the pile, the allowable stresses specified in this chapter are permitted to be increased where satisfactory substantiating data are submitted.

**1807.2.12 Settlement analysis.** The settlement of piers, individual piles or groups of piles shall be estimated...
based on approved methods of analysis. The predicted settlement shall neither cause harmful distortion of, or instability in, the structure, nor cause any stresses to exceed allowable values.

1807.2.13 Pre-excavation. The use of jetting, augering or other methods of pre-excavation shall be subject to the approval of the building official. Where permitted, pre-excavation shall be carried out in the same manner as used for piers or piles subject to load tests and in such a manner that will not impair the carrying capacity of the piers or piles already in place or damage adjacent structures. Pile tips shall be driven below the pre-excavated depth until the required resistance or penetration is obtained.

1807.2.14 Installation sequence. Piles shall be installed in such sequence as to avoid compacting the surrounding soil to the extent that other piles cannot be installed properly, and to prevent ground movements that are capable of damaging adjacent structures.

1807.2.15 Use of vibratory drivers. Vibratory drivers shall only be used to install piles where the pile load capacity is verified by load tests in accordance with Section 1807.2.8.3. The installation of production piles shall be controlled according to power consumption, rate of penetration or other approved means that ensure pile capacities equal or exceed those of the test piles.

1807.2.16 Pile driveability. Pile cross sections shall be of sufficient size and strength to withstand driving stresses without damage to the pile, and to provide sufficient stiffness to transmit the required driving forces.

1807.2.17 Protection of pile materials. Where boring records or site conditions indicate possible deleterious action on pier or pile materials because of soil constituents, changing water levels or other factors, the pier or pile materials shall be adequately protected by materials, methods or processes approved by the building official. Protective materials shall be applied to the piles so as not to be rendered ineffective by driving. The effectiveness of such methods or processes for the particular purpose shall have been thoroughly established by satisfactory service records or other evidence that demonstrates the effectiveness of such protective measures.

1807.2.18 Use of existing piers or piles. Piers or piles left in place where a structure has been demolished shall not be used for the support of new construction unless satisfactory evidence is submitted to the building official, which indicates that the piers or piles are sound and meet the requirements of this code. Such piers or piles shall be load tested or redriven to verify their capacities. The design load applied to such piers or piles shall be the lowest allowable load as determined by tests or redriving data.

1807.2.19 Heaved piles. Piles that have heaved during the driving of adjacent piles shall be redriven as necessary to develop the required capacity and penetration, or the capacity of the pile shall be verified by load tests in accordance with Section 1807.2.8.3.

1807.2.20 Identification. Pier or pile materials shall be identified for conformity to the specified grade with this identity maintained continuously from the point of manufacture to the point of installation or shall be tested by an approved agency to determine conformity to the specified grade. The approved agency shall furnish an affidavit of compliance to the building official.

1807.2.21 Pier or pile location plan. A plan showing the location and designation of piers or piles by an identification system shall be filed with the building official prior to installation of such piers or piles. Detailed records for piers or individual piles shall bear an identification corresponding to that shown on the plan.

1807.2.22 Special inspection. Special inspections in accordance with Sections 1704.8 and 1704.9 shall be provided for piles and piers, respectively.

1807.2.23 Seismic design of piers or piles.

1807.2.23.1 Seismic Design Category C. Where a structure is assigned to Seismic Design Category C in accordance with Section 1616, the following shall apply. Individual pile caps, piers or piles shall be interconnected by ties. Ties shall be capable of carrying, in tension and compression, a force equal to the product of the larger pile cap or column load times the seismic coefficient $S_{DS}$ divided by 10 unless it can be demonstrated that equivalent restraint is provided by reinforced concrete beams within slabs-on-grade or reinforced concrete slabs-on-grade or confinement by competent rock, hard cohesive soils, or very dense granular soils.

Exception: Piers supporting foundation walls, isolated interior posts detailed so the pier is not subject to lateral loads, lightly loaded exterior decks and patios, of Group R, Division 3 and Group U, Division 1 occupancies not exceeding two stories of light-frame construction, are not subject to interconnection if it can be shown the soils are of adequate stiffness, subject to the approval of the building official.
1807.2.23.1.1 Connection to pile cap. Concrete piles and concrete-filled steel pipe piles shall be connected to the pile cap by embedding the pile reinforcement or field-placed dowels anchored in the concrete pile in the pile cap for a distance equal to the development length. For deformed bars, the development length is the full development length for compression or tension without reduction in length for excess area. Alternative measures for laterally confining concrete and maintaining toughness and ductile-like behavior at the top of the pile will be permitted provided the design is such that any hinging occurs in the confined region. Ends of hoops or ties shall be terminated with 135-degree (2.35 rad) hooks into the confined concrete core. The minimum transverse steel ratio for confinement shall not be less than one-half of that required for columns.

For resistance to uplift forces, anchorage of steel pipe, concrete-filled steel pipe or H piles to the pile cap shall be made by means other than concrete bond to the bare steel section.

Splices of pile segments shall develop the full strength of the pile.

1807.2.23.1.2 Design details. Pier or pile moments, shears and lateral deflections used for design shall be established considering the nonlinear interaction of the shaft and soil, as recommended by a registered design professional. Where the ratio of the depth of embedment of the pile-to-pile diameter or width is less than or equal to six, the pile may be assumed to be rigid.

Pier or pile group effects from soil on lateral pile capacity shall be considered where pile center-to-center spacing in the direction of lateral force is less than 8 pile diameters. Pile group effects on vertical capacity shall be considered where pile center-to-center spacing is less than 3 pile diameters.

Where a minimum length for reinforcement or the extent of closely spaced confinement reinforcement is specified at the top of the pier or pile, provisions shall be made so that those specified lengths or extents are maintained after pier or pile cut-off.

1807.2.23.2 Seismic Design Category D, E or F. Where a structure is assigned to Seismic Design Category D, E or F in accordance with Section 1616, the requirements for Seismic Design Category C given in Section 1807.2.23.1 shall be met, in addition to the following. Provisions of ACI 318, Section 21.8.4, shall apply when not in conflict with the provisions of Sections 1807 through 1811. Concrete shall have a specified compressive strength of not less than 3,000 psi (20.68 MPa) at 28 days.

Exception: Group R or Group U Occupancies of light-frame construction and two stories or less in height are permitted to use concrete with a specified compressive strength of not less than 2,500 psi (17.2 MPa) at 28 days.

1807.2.23.2.1 Design details. Piers or piles shall be designed and constructed to withstand maximum imposed curvatures from earthquake ground motions and structure response. Curvatures shall include free-field soil strains modified for soil-pile-structure interaction coupled with pier or pile deformations induced by lateral pier or pile resistance to structure seismic forces. Concrete piers or piles on Site Class E or F sites, as determined in Section 1615.1.1, shall be designed and detailed in accordance with requirements for concrete special moment frames (see Table 1617.6 for reference) within 7 pile diameters of the pile cap and the interfaces of soft to medium stiff clay or liquefiable strata. For precast prestressed concrete piles, detailing provisions as given in Sections 1808.2.3.2.1 and 1808.2.3.2.2 shall apply.

Grade beams shall be designed as beams in accordance with ACI 318, Chapter 21. When grade beams have the capacity to resist the forces from the load combinations in Section 1617.1.2, they need not conform to ACI 318, Chapter 21.

1807.2.23.2.2 Connection to pile cap. Design of anchorage of piles into the pile cap shall consider the combined effect of uplift forces and fixity to the pile cap. Anchorage shall develop a minimum of 25 percent of the strength of the pile in tension. For piles required to resist uplift forces or provide rotational restraint, anchorage into the pile cap shall be capable of developing, at a minimum, the lesser of the following:

1. The tensile strength of the longitudinal reinforcement in a concrete pile or the tensile strength of a steel pile.
2. 1.3 times the pile uplift capacity in the soil.

1807.2.23.2.3 Flexural strength. Where the vertical lateral-force-resisting elements are columns, the grade beam or pile cap flexural strengths shall exceed the column flexural strength.

Batter piles and their connection shall be capable of resisting forces from the load combinations of Section 1605.4.
SECTION 1808
DRIVEN PILE FOUNDATIONS

1808.1 Timber piles. Timber piles shall be designed in accordance with the AFPA NDS.


1808.1.2 Preservative treatment. Timber piles used to support permanent structures shall be treated in accordance with this section unless it is established that the tops of the untreated timber piles will be below the lowest ground water level assumed to exist during the life of the structure. Preservative and minimum final retention shall be in accordance with AWPA C3 for round timber piles, and AWPA C24 for sawn timber piles. Preservative-treated timber piles shall be subject to a quality control program administered by an approved agency. Pile cutoffs shall be treated in accordance with AWPA M 4.

1808.1.3 End-supported piles. Any sudden decrease in driving resistance of an end-supported timber pile shall be investigated with regard to the possibility of damage. If the sudden decrease in driving resistance cannot be correlated to load-bearing data, the pile shall be removed for inspection or rejected.

1808.2 Precast concrete piles.

1808.2.1 General. The materials, reinforcement and installation of precast concrete piles shall conform to Sections 1808.2.1.1 through 1808.2.1.4.

1808.2.1.1 Design and manufacture. Piles shall be designed and manufactured in accordance with accepted engineering practice to resist all stresses induced by handling, driving and service loads.

1808.2.1.2 Minimum dimension. The minimum lateral dimension shall be 8 inches (203 mm). Corners of square piles shall be chamfered.

1808.2.1.3 Reinforcement. Longitudinal steel shall be arranged in a symmetrical pattern and shall be laterally tied with steel ties or wire spiral spaced not more than 4 inches (102 mm) apart, center-to-center, for a distance of 2 feet (610 mm) from the ends of the pile; and not more than 6 inches (152 mm) elsewhere except that at the ends of each pile, the first five ties or spirals shall be spaced 1 inch (25.4 mm) center to center. The gage of ties and spirals shall be as follows:

For piles having a diameter of 16 inches (406 mm) or less, wire shall not be smaller than 0.22 inch (5.6 mm) (No. 5 gage).

For piles having a diameter of more than 16 inches (406 mm) and less than 20 inches (508 mm), wire shall not be smaller than 0.238 inch (6 mm) (No. 4 gage).

For piles having a diameter of 20 inches (508 mm) and larger, wire shall not be smaller than 1/4 inch (6.4 mm) round or 0.259 inch (6.6 mm) (No. 3 gage).

1808.2.2 Precast nonprestressed piles. Precast nonprestressed concrete piles shall conform to Sections 1808.2.2.1 through 1808.2.2.5.

1808.2.2.1 Materials. Concrete shall have a 28-day specified compressive strength ($f'_{c}$) of not less than 3,000 psi (20.68 MPa).

1808.2.2.2 Minimum reinforcement. The minimum amount of longitudinal reinforcement shall be 0.8 percent of the concrete section and shall consist of at least four bars.

1808.2.2.2.1 Seismic reinforcement in Seismic Design Category C. Where a structure is assigned to Seismic Design Category C in accordance with Section 1616, the following shall apply. Longitudinal reinforcement shall be provided for precast concrete piles with a minimum steel ratio of 0.01. The longitudinal reinforcing shall be confined with closed ties or equivalent spirals of a minimum of a 1/4 inch (6.4 mm) diameter. Ties or equivalent spirals shall be provided at a maximum 8-bar-diameter spacing with a maximum spacing of 6 inches (152 mm). Reinforcement including ties shall be full length.

1808.2.2.2.2 Seismic reinforcement in Seismic Design Category D, E or F. Where a structure is assigned to Seismic Design Category D, E or F in accordance with Section 1616, the requirements for Seismic Design Category C in Section 1808.2.2.2.1 shall be met. Ties or equivalent spirals shall be provided at a maximum 6-longitudinal-bar-diameter spacing not to exceed a maximum of 4 inches (102 mm) on center. In addition, ties in precast concrete piles shall be provided for at least the top half of the pile.
1808.2.2.3 Allowable stresses. The allowable compressive stress in the concrete shall not exceed 33 percent of the 28-day specified compressive strength ($f'_c$) applied to the gross cross-sectional area of the pile. The allowable compressive stress in the reinforcing steel shall not exceed 40 percent of the yield strength of the steel ($f_y$) or a maximum of 30,000 psi (207 MPa). The allowable tensile stress in the reinforcing steel shall not exceed 50 percent of the yield strength of the steel ($f_y$) or a maximum of 24,000 psi (165 MPa).

1808.2.2.4 Installation. A precast concrete pile shall not be driven before the concrete has attained a compressive strength of at least 75 percent of the 28-day specified compressive strength ($f'_c$), but not less than the strength sufficient to withstand handling and driving forces.

1808.2.2.5 Concrete cover. Reinforcement for piles that are not manufactured under plant conditions shall have a concrete cover of not less than 2 inches (51 mm).

Reinforcement for piles manufactured under plant control conditions shall have a concrete cover of not less than 1 1/4 inches (32 mm) for No. 5 bars and smaller, and not less than 1 1/2 inches (38 mm) for No. 6 through No. 11 bars except that longitudinal bars spaced less than 1 1/2 inches (38 mm) clear distance apart shall be considered bundled bars for which the minimum concrete cover shall be equal to that for the equivalent diameter of the bundled bars.

Reinforcement for piles exposed to sea water shall have a concrete cover of not less than 3 inches (76 mm).

1808.2.3 Precast prestressed piles. Precast prestressed concrete piles shall conform to the requirements of Sections 1808.2.3.1 through 1808.2.3.5.

1808.2.3.1 Materials. Prestressing steel shall conform to ASTM A 416. Concrete shall have a 28-day specified compressive strength ($f'_c$) of not less than 5,000 psi (34.48 MPa).

1808.2.3.2 Design. Precast prestressed piles shall be designed to resist stresses induced by handling and driving as well as by loads. The effective prestress in the pile shall not be less than 400 psi (2.76 MPa) for piles up to 30 feet (9144 mm) in length, 550 psi (3.79 MPa) for piles up to 50 feet (15 240 mm) in length, and 700 psi (4.83 MPa) for piles greater than 50 feet (15 240 mm) in length.

Effective prestress shall be based on an assumed loss of 30,000 psi (207 MPa) in the prestressing steel.

The tensile stress in the prestressing steel shall not exceed the values specified in ACI 318.

1808.2.3.2.1 Design in Seismic Design Category C. Where a structure is assigned to Seismic Design Category C in accordance with Section 1616, the following shall apply. The minimum volumetric ratio of spiral reinforcement in the ductile region shall be equal to 0.007. The spiral reinforcement shall not be less than the amount required by the following formula:

$$\rho_s = 0.12\frac{f'_c}{f_{yh}}$$

(Equation 18-4)

where:

- $f'_c \leq 6,000$ psi (41.4 MPa).
- $f_{yh}$ Yield strength of spiral reinforcement $\leq 85,000$ psi (586 MPa).
- $\rho_s$ = Spiral reinforcement index (vol. spiral/ vol. core).

The pile cap connection by means of dowels as indicated in Section 1807.2.23.1 is permitted. Pile cap connection by means of developing pile reinforcing strand is permitted provided that the pile reinforcing strand results in a ductile connection.

1808.2.3.2.2 Design in Seismic Design Category D, E or F. Where a structure is assigned to Seismic Design Category D, E or F in accordance with Section 1616, the requirements for Seismic Design Category C in Section 1808.2.3.2.1 shall be met, in addition to the following:

1. Requirements in ACI 318, Chapter 21, need not apply.

2. Where the total pile length in the soil is 35 feet (10 668 mm) or less, the lateral transverse reinforcement in the ductile region shall occur through the length of the pile. Where the pile length exceeds 35 feet (10 668 mm), the ductile pile region shall be taken as the greater of 35 feet (10 668 mm) or the distance from the underside of the pile cap to the point of zero curvature plus three times the least pile dimension.

3. In the ductile region, the center-to-center spacing of the spirals or hoop reinforcement shall not exceed one-fifth of the least pile dimension, six times the diameter of the longitudinal strand, or 8 inches (203 mm), whichever is smaller.

4. Spiral reinforcement shall be spliced by lapping one full turn by welding or by the use of a mechanical connector. Where spiral
reinforcement is lap spliced, the ends of the spiral shall terminate in a seismic hook in accordance with ACI 318, except that the bend shall not be less than 135 degrees (2.35 rad). Welded splices and mechanical connectors shall comply with Section 12.14.3 of ACI 318.

5. Where the transverse reinforcement consists of circular spirals, the volumetric ratio of spiral transverse reinforcement in the ductile region shall comply with the following:

\[ \rho_s = 0.25(\frac{f'c}{f_{sh}})(A_g/A_{ch} - 1.0)[0.5 + \frac{1.4P}{(f'cA_g)}] \]  
\[ \text{(Equation 18-5)} \]

but not less than

\[ \rho_s = 0.12(\frac{f'c}{f_{sh}})[0.5 + \frac{1.4P}{(f'cA_g)}] \]  
\[ \text{(Equation 18-6)} \]

and need not exceed

\[ \rho_s = 0.021 \]  
\[ \text{(Equation 18-7)} \]

where:

- \( A_g \) = Pile cross-sectional area, square inch (mm²).
- \( A_{ch} \) = Core area defined by spiral outside diameter, square inch (mm²).
- \( f'c \) =\leq 6,000 psi (41.4 MPa).
- \( f_{sh} \) = Yield strength of spiral reinforcement \( \leq 85,000 \text{ psi (586 MPa).} \)
- \( P \) = Axial load on pile, pounds (kN), as determined from Formulas 16-5 and 16-6.
- \( \rho_s \) = Volumetric ratio (vol. spiral/ vol. core).

This required amount of spiral reinforcement is permitted to be obtained by providing an inner and outer spiral.

6. When transverse reinforcement consists of rectangular hoops and cross-ties, the total cross-sectional area of lateral transverse reinforcement in the ductile region with spacings, and perpendicular to dimension, \( h_c \), shall conform to:

\[ A_{ch} = 0.33h_c(\frac{f'c}{f_{sh}})(A_g/A_{ch} - 1.0)[0.5 + \frac{1.4P}{(f'cA_g)}] \]  
\[ \text{(Equation 18-8)} \]

but not less than

\[ A_{ch} = 0.12h_c(\frac{f'c}{f_{sh}})(A_g/A_{ch} - 1.0)[0.5 + \frac{1.4P}{(f'cA_g)}] \]  
\[ \text{(Equation 18-9)} \]

where:

- \( f_{sh} \) = \leq 70,000 psi (483 MPa).
- \( h_c \) = Cross-sectional dimension of pile core measured center to center of hoop reinforcement, inch (mm).
- \( s \) = Spacing of transverse reinforcement measured along length of pile, inch (mm).
- \( A_{ch} \) = Cross sectional area of transverse reinforcement, square inch (mm²).

The hoops and cross-ties shall be equivalent to deformed bars not less than No. 3 in size. Rectangular hoop ends shall terminate at a corner with seismic hooks.

1808.2.3.3 Allowable stresses. The maximum allowable design compressive stress, \( f_c \), in concrete shall be determined as follows:

\[ f_c = 0.33f'c - 0.27f_{pc} \]  
\[ \text{(Equation 18-10)} \]

where:

- \( f'c \) = The 28-day specified compressive strength of the concrete.
- \( f_{pc} \) = The effective prestress stress on the gross section.

1808.2.3.4 Installation. A prestressed pile shall not be driven before the concrete has attained a compressive strength of at least 75 percent of the 28-day specified compressive strength \( (f'c) \), but not less than the strength sufficient to withstand handling and driving forces.

1808.2.3.5 Concrete cover. Prestressing steel and pile reinforcement shall have a concrete cover of not less than 11/4 inches (32 mm) for square piles of 12 inches (305 mm) or smaller size and 11/2 inches (38 mm) for larger piles, except that for piles exposed to sea water, the minimum protective concrete cover shall not be less than 21/2 inches (64 mm).

1808.3 Structural steel piles. Structural steel piles shall conform to the requirements of Sections 1808.3.1 through 1808.3.5.

1808.3.1 Materials. Structural steel piles, steel pipe and fully welded steel piles fabricated from plates shall conform to ASTM A 36, A 252, A 283, A 572, A 588 or A 913.
1808.3.2 Allowable stresses. The allowable axial stresses shall not exceed 35 percent of the minimum specified yield strength ($F_y$).

Exception: Where justified in accordance with Section 1807.2.10, the allowable axial stress is permitted to be increased above $0.35F_y$, but shall not exceed $0.5F_y$.

1808.3.3 Dimensions of H-piles. Sections of H-piles shall comply with the following:

1. The flange projections shall not exceed 14 times the minimum thickness of metal in either the flange or the web and the flange widths shall not be less than 80 percent of the depth of the section.
2. The nominal depth in the direction of the web shall not be less than 8 inches (203 mm).
3. Flanges and web shall have a minimum nominal thickness of $\frac{1}{8}$ inch (9.5 mm).

1808.3.4 Dimensions of steel pipe piles. Steel pipe piles driven open ended shall have a nominal outside diameter of not less than 8 inches (203 mm). The pipe shall have a minimum of 0.34 square inch (219 mm$^2$) of steel in cross section to resist each 1,000 foot-pounds (1356 N·m) of pile hammer energy or the equivalent strength for steels having a yield strength greater than 35,000 psi (241 MPa). Where pipe wall thickness less than 0.188 inch (4.8 mm) is driven open ended, a suitable cutting shoe shall be provided.

1808.3.5 Design in Seismic Design Category D, E or F. Where a structure is assigned to Seismic Design Category D, E or F in accordance with Section 1616, I-shaped sections shall have an unsupported flange to thickness ratio not to exceed

$$52\sqrt{F_y} \text{ (For SI: } 0.317\sqrt{E/F_y}). \quad \text{(Equation 18-11)}$$

Circular sections shall have an outside wall diameter to wall thickness ratio not to exceed

$$1,300\sqrt{F_y} \text{ (For SI: } 7.63\sqrt{E/F_y}). \quad \text{(Equation 18-12)}$$

SECTION 1809
CAST-IN-PLACE CONCRETE PILE FOUNDATIONS

1809.1 General. The materials, reinforcement and installation of cast-in-place concrete piles shall conform to Sections 1809.1.1 through 1809.1.3.

1809.1.1 Materials. Concrete shall have a 28-day specified compressive strength ($f'_c$) of not less than 2,500 psi (17.24 MPa). Where concrete is placed through a funnel hopper at the top of the pile, the concrete mix shall be designed and proportioned so as to produce a cohesive workable mix having a slump of not less than 4 inches (102 mm) and not more than 6 inches (152 mm). Where concrete is to be pumped, the mix design including slump shall be adjusted to produce a pumpable concrete.

1809.1.2 Reinforcement. Except for steel dowels embedded 5 feet (1524 mm) or less in the pile and as provided in Section 1809.3.4, reinforcement where required shall be assembled and tied together and shall be placed in the pile as a unit before the reinforced portion of the pile is filled with concrete except in augered uncased cast-in-place piles. Tied reinforcement in augered uncased cast-in-place piles shall be placed after piles are concreted, while the concrete is still in a semifluid state.

1809.1.2.1 Reinforcement in Seismic Design Category C. Where a structure is assigned to Seismic Design Category C in accordance with Section 1616, the following shall apply. A minimum reinforcement ratio of 0.0025 shall be provided for uncased cast-in-place concrete drilled piles, drilled piers, or caissons in the top one-third of the pile length or a minimum length of 10 feet (3048 mm) below the ground. The minimum reinforcement ratio, but no less than that ratio required by rational analysis, shall be continued throughout the flexural length of the pile. There shall be a minimum of four bars with closed ties (or equivalent spirals) of a minimum $\frac{1}{4}$ inch (6.4 mm) diameter provided at 16-longitudinal-bar-diameter maximum spacing. A maximum spacing of 6 inch (152 mm) or 8-longitudinal-bar-diameters, whichever is less, shall be provided within a distance equal to two times the least pile dimension of the bottom of the pile cap.

1809.1.2.2 Reinforcement in Seismic Design Category D, E or F. Where a structure is assigned to Seismic Design Category D, E or F in accordance with Section 1616, the requirements for Seismic Design Category C given above shall be met, in addition to the following. A minimum reinforcement ratio of 0.005 shall be provided for uncased cast-in-place concrete piles, drilled piers, or caissons in the top one-half of the pile length or a minimum length of 10 feet (3048 mm) below ground. The minimum reinforcement ratio, but no less than that ratio required by rational analysis, shall be continued throughout the flexural length of the pile. There shall be a minimum of four bars with closed ties or equivalent spirals provided at 6-longitudinal-bar-diameter maximum spacing with a maximum spacing of 4 inches (102 mm) within seven times the least pile dimension of the bottom of the pile cap in Site Class E or F and at the interfaces of soft to medium stiff clay or liquefiable...
strata, and three times the least pile dimension at all other sites. Tie spacing throughout the remainder of the concrete section shall not exceed 12-longitudinal-bar-diameters, one-half the least dimension of the section, nor 12 inches (305 mm). Ties shall be a minimum of No. 3 bars for piles with a least dimension up to 20 inches (508 mm), and No. 4 bars for larger piles.

1809.1.3 Concrete placement. Concrete shall be placed in such a manner as to ensure the exclusion of any foreign matter and to secure a full-sized shaft. Concrete shall not be placed through water except where a tremie or other approved method is used. When depositing concrete from the top of the pile, the concrete shall not be chuted directly into the pile but shall be poured in a rapid and continuous operation through a funnel hopper centered at the top of the pile.

1809.2 Enlarged base piles. Enlarged base piles shall conform to the requirements of Sections 1809.2.1 through 1809.2.5.

1809.2.1 Materials. The maximum size for coarse aggregate for concrete shall be $\frac{3}{4}$ inch (19.1 mm). Concrete to be compacted shall have a zero slump.

1809.2.2 Allowable stresses. The maximum allowable design compressive stress for concrete not placed in a permanent steel casing shall be 25 percent of the 28-day specified compressive strength ($f'_{c}$). Where the concrete is placed in a permanent steel casing, the maximum allowable concrete stress shall be 33 percent of the 28-day specified compressive strength ($f'_{c}$).

1809.2.3 Installation. Enlarged bases formed either by compacting concrete or driving a precast base shall be formed in or driven into granular soils. Piles shall be constructed in the same manner as successful prototype test piles driven for the project. Pile shafts extending through peat or other organic soil shall be encased in a permanent steel casing. Where a cased shaft is used, the shaft shall be adequately reinforced to resist column action or the annular space around the pile shaft shall be filled sufficiently to re-establish the lateral support of the soil. Where pile heave occurs, the pile shall be replaced unless it is demonstrated that the pile is undamaged and capable of carrying twice its design load.

1809.2.4 Load-bearing capacity. Pile load-bearing capacity shall be verified by load tests in accordance with Section 1807.2.8.3.

1809.2.5 Concrete cover. The minimum concrete cover shall be $2\frac{1}{2}$ inches (64 mm) for uncased shafts and 1 inch (25 mm) for cased shafts.

1809.3 Drilled or augered uncased piles. Drilled or augered uncased piles shall conform to Sections 1809.3.1 through 1809.3.5.

1809.3.1 Allowable stresses. The allowable design stress in the concrete of drilled uncased piles shall not exceed 33 percent of the 28-day specified compressive strength ($f'_{c}$). The allowable design stress in the concrete of augered cast-in-place piles shall not exceed 25 percent of the 28-day specified compressive strength ($f'_{c}$). The allowable compressive stress of reinforcement shall not exceed 34 percent of the yield strength of the steel or 25,500 psi (175.8 MPa).

1809.3.2 Dimensions. The pile length shall not exceed 30 times the average diameter. The minimum diameter shall be 12 inches (305 mm).

Exception: The length of the pile is permitted to exceed 30 times the diameter, provided that the design and installation of the pile foundation is under the direct supervision of a registered design professional knowledgeable in the field of soil mechanics and pile foundations. The registered design professional shall certify to the building official that the piles were installed in compliance with the approved design.

1809.3.3 Installation. Where pile shafts are formed through unstable soils and concrete is placed in an open-drilled hole, a steel liner shall be inserted in the hole prior to placing the concrete. Where the steel liner is withdrawn during concreting, the level of concrete shall be maintained above the bottom of the liner at a sufficient height to offset any hydrostatic or lateral soil pressure.

Where concrete is placed by pumping through a hollow-stem auger, the auger shall be permitted to rotate in a clockwise direction during withdrawal. The auger shall be withdrawn in a continuous manner in increments of about 12 inches (305 mm) each. Concreting pumping pressures shall be measured and shall be maintained high enough at all times to offset hydrostatic and lateral earth pressures. Concrete volumes shall be measured to ensure that the volume of concrete placed in each pile is equal to or greater than the theoretical volume of the hole created by the auger. Where the installation process of any pile is interrupted or a loss of concreting pressure occurs, the pile shall be redrilled to 5 feet (1524 mm) below the elevation of the tip of the auger when the installation was interrupted or concrete pressure was lost and reformed. Augered cast-in-place piles shall not be installed within 6 pile diameters center-to-center of a pile filled with concrete less than 12 hours old, unless approved by the building official. If the concrete level in any completed pile drops during installation of an adjacent pile, the pile shall be replaced.
1809.3.4 Reinforcement. For piles installed with a hollow-stem auger, where full length longitudinal steel reinforcement is placed without lateral ties, the reinforcement shall be placed through ducts in the auger prior to filling the pile with concrete. All pile reinforcement shall have a concrete cover of not less than 2 1/2 inches (64 mm).

Exception: Where physical constraints do not allow the placement of the longitudinal reinforcement prior to filling the pile with concrete or where partial length longitudinal reinforcement is placed without lateral ties, the reinforcement is allowed to be placed after the piles are completely concreted but while concrete is still in a semifluid state.

1809.3.5 Reinforcement in Seismic Design Category C, D, E or F. Where a structure is assigned to Seismic Design Category C, D, E or F in accordance with Section 1616, the corresponding requirements of Sections 1809.1.2.1 and 1809.1.2.2 shall be met.

1809.4 Driven uncased piles. Driven uncased piles shall conform to Sections 1809.4.1 through 1809.4.4.

1809.4.1 Allowable stresses. The allowable design stress in the concrete shall not exceed 25 percent of the 28-day specified compressive strength ($f'_c$) applied to a cross-sectional area not greater than the inside area of the drive casing or mandrel.

1809.4.2 Dimensions. The pile length shall not exceed 30 times the average diameter. The minimum diameter shall be 12 inches (305 mm).

Exception: The length of the pile is permitted to exceed 30 times the diameter, provided that the design and installation of the pile foundation is under the direct supervision of a registered design professional knowledgeable in the field of soil mechanics and pile foundations. The registered design professional shall certify to the building official that the piles were installed in compliance with the approved design.

1809.4.3 Installation. Piles shall not be driven within 6 pile diameters center-to-center in granular soils or within one-half the pile length in cohesive soils of a pile filled with concrete less than 48 hours old unless approved by the building official. If the concrete surface in any completed pile rises or drops, the pile shall be replaced. Piles shall not be installed in soils that could cause pile heave.

1809.4.4 Concrete cover. Pile reinforcement shall have a concrete cover of not less than 2 1/2 inches (64 mm), measured from the inside face of the drive casing or mandrel.

1809.5 Steel-cased piles. Steel-cased piles shall comply with the requirements of Sections 1809.5.1 through 1809.5.4.

1809.5.1 Materials. Pile shells or casings shall be of steel and shall be sufficiently strong to resist collapse and sufficiently watertight to exclude any foreign materials during the placing of concrete. Steel shells shall have a sealed tip with a diameter of not less than 8 inches (203 mm).

1809.5.2 Allowable stresses. The allowable design compressive stress in the concrete shall not exceed 33 percent of the 28-day specified compressive strength ($f'_c$). The allowable concrete compressive stress shall be 0.40 ($f'_c$) for that portion of the pile meeting the conditions specified in Sections 1809.5.2.1 through 1809.5.2.4.

1809.5.2.1 Shell thickness. The thickness of the steel shell shall not be less than manufacturer's standard gage No. 14 gage (0.068 inch) (1.75 mm) minimum.

1809.5.2.2 Shell type. The shell shall be seamless or shall be provided with seams of strength equal to the basic material and be of a configuration that will provide confinement to the cast-in-place concrete.

1809.5.2.3 Strength. The ratio of steel yield strength ($f_y$) to 28-day specified compressive strength ($f'_c$) shall not be less than six.

1809.5.2.4 Diameter. The nominal pile diameter shall not be greater than 16 inches (406 mm).

1809.5.3 Installation. Piles shall have steel shells that are mandrel-driven their full length in contact with the surrounding soil, left permanently in place and filled with concrete.

Steel shells shall be driven in such order and with such spacing as to ensure against distortion of or injury to piles already in place. A pile shall not be driven within 4 1/2 average pile diameters of a pile filled with concrete less than 24 hours old unless approved by the building official. Concrete shall not be placed in steel shells within heave range of driving.

1809.5.4 Reinforcement. Reinforcement shall not be placed within 1 inch (25 mm) of the steel shell. Reinforcing shall be required for unsupported pile lengths or where the pile is designed to resist uplift or unbalanced lateral loads.

1809.5.4.1 Seismic reinforcement. Where a structure is assigned to Seismic Design Category C, D, E or F in accordance with Section 1616, the reinforcement...
requirements for drilled or augered uncased piles in Section 1809.3.5 shall be met.

**Exception:** A spiral welded metal-casing of a thickness not less than manufacturer’s standard gage No. 14 gage (0.068 inch) is permitted to provide concrete confinement in lieu of the closed ties or equivalent spirals required in an uncased concrete pile. Where used as such, the metal casing shall be protected against possible deleterious action due to soil constituents, changing water levels, or other factors indicated by boring records of site conditions.

### 1809.6 Concrete-filled steel pipe and tube piles.
Concrete-filled steel pipe and tube piles shall conform to the requirements of Sections 1809.6.1 through 1809.6.5.

#### 1809.6.1 Materials.
Steel pipe and tube sections used for piles shall conform to ASTM A 252 or A 283. Concrete shall conform to Section 1809.1.1. The maximum coarse aggregate size shall be 3/4 inch (19.1 mm).

#### 1809.6.2 Allowable stresses.
The allowable design compressive stress in the concrete shall not exceed 33 percent of the 28-day specified compressive strength (f'c). The allowable design compressive stress in the steel shall not exceed 35 percent of the minimum specified yield strength of the steel (Fy), provided Fy shall not be assumed greater than 36,000 psi (248 MPa) for computational purposes.

**Exception:** Where justified in accordance with Section 1807.2.10, the allowable stresses are permitted to be increased to 0.50 Fy.

#### 1809.6.3 Minimum dimensions.
Piles shall have a nominal outside diameter of not less than 8 inches (203 mm) and a minimum wall thickness in accordance with Section 1808.3.4. For mandrel-driven pipe piles, the minimum wall thickness shall be 1/10 inch (2.5 mm).

#### 1809.6.4 Reinforcement.
Reinforcement steel shall conform to Section 1809.1.2. Reinforcement shall not be placed within 1 inch (25 mm) of the steel casing.

##### 1809.6.4.1 Seismic reinforcement.
Where a structure is assigned to Seismic Design Category C, D, E or F in accordance with Section 1616, the following shall apply. Minimum reinforcement no less than 0.01 times the cross-sectional area of the pile concrete shall be provided in the top of the pile with a length equal to two times the required cap embedment anchorage into the pile cap, but not less than the tension development length of the reinforcement. The wall thickness of the steel pipe shall not be less than 3/16 inch (5 mm).

#### 1809.6.5 Placing concrete.
The placement of concrete shall conform to Section 1809.1.3.

### 1809.7 Caisson piles.
Caisson piles shall conform to the requirements of Sections 1809.7.1 through 1809.7.6.

#### 1809.7.1 Construction.
Caisson piles shall consist of a shaft section of concrete-filled pipe extending to bedrock with an uncased socket drilled into the bedrock and filled with concrete. The caisson pile shall have a full-length structural steel core or a stub core installed in the rock socket and extending into the pipe portion a distance equal to the socket depth.

#### 1809.7.2 Materials.
Pipe and steel cores shall conform to the material requirements in Section 1808.3. Pipes shall have a minimum wall thickness of 3/8 inch (9.5 mm) and shall be fitted with a suitable steel-driving shoe welded to the bottom of the pipe. Concrete shall have a 28-day specified compressive strength (f'c) of not less than 4,000 psi (27.58 MPa). The concrete mix shall be designed and proportioned so as to produce a cohesive workable mix with a slump of 4 inches (102 mm) to 6 inches (152 mm).

#### 1809.7.3 Design.
The depth of the rock socket shall be sufficient to develop the full load-bearing capacity of the caisson pile with a minimum safety factor of two, but the depth shall not be less than the outside diameter of the pipe. The design of the rock socket is permitted to be predicated on the sum of the allowable load-bearing pressure on the bottom of the socket plus bond along the sides of the socket. The minimum outside diameter of the caisson pile shall be 18 inches (457 mm), and the diameter of the rock socket shall be approximately equal to the inside diameter of the pile.

#### 1809.7.4 Structural core.
The gross cross-sectional area of the structural steel core shall not exceed 25 percent of the gross area of the caisson. The minimum clearance between the structural core and the pipe shall be 2 inches (51 mm). Where cores are to be spliced, the ends shall be milled or ground to provide full contact and shall be full-depth welded.

#### 1809.7.5 Allowable stresses.
The allowable design compressive stresses shall not exceed the following: concrete, 0.33 f'c; steel pipe, 0.35 Fy; and structural steel core, 0.50 Fy.

#### 1809.7.6 Installation.
The rock socket and pile shall be thoroughly cleaned of foreign materials before filling with concrete. Steel cores shall be bedded in cement grout at the base of the rock socket. Concrete shall not be
placed through water except where a tremie or other approved method is used.

SECTION 1810
COMPOSITE PILES

1810.1 General. Composite piles shall conform to the requirements of Sections 1810.2 through 1810.5.

1810.2 Design. Composite piles consisting of two or more approved pile types shall be designed to meet the conditions of installation.

1810.3 Limitation of load. The maximum allowable load shall be limited by the capacity of the weakest section incorporated in the pile.

1810.4 Splices. Splices between concrete and steel or wood sections shall be designed to prevent separation both before and after the concrete portion has set, and to ensure the alignment and transmission of the total pile load. Splices shall be designed to resist uplift caused by upheaval during driving of adjacent piles, and shall develop the full compressive strength and not less than 50 percent of the tension and bending strength of the weaker section.

1810.5 Seismic reinforcement. Where a structure is assigned to Seismic Design Category C, D, E or F in accordance with Section 1616, the following shall apply. Where concrete and steel are used as part of the pile assembly, the concrete reinforcement shall comply with that given in Sections 1809.1.2.1 and 1809.1.2.2 or the steel section shall comply with Section 1809.6.4.1 or 1808.3.5.

SECTION 1811
PIER FOUNDATIONS

1811.1 General. Isolated and multiple piers used as foundations shall conform to the requirements of Sections 1811.2 through 1811.10, as well as the applicable provisions of Section 1807.2.

1811.2 Lateral dimensions and height. The minimum dimension of isolated piers used as foundations shall be 2 feet (610 mm), and the height shall not exceed 12 times the least horizontal dimension.

Exceptions:
1. The height limitations shall not apply to buildings of Group R, Division 3 and Group U, Division 1 occupancies not exceeding two stories of light-frame construction, subject to the approval of the building official.
2. The lateral dimension and height limitations shall not apply where the piers are constructed of reinforced concrete or structural steel, or are entirely encased in a steel shell at least ⅛ inch (6.4 mm) thick.
3. Where the surrounding foundation materials furnish adequate lateral support, heights greater than herein specified are permitted, subject to the approval of the building official.

1811.3 Materials. Concrete shall have a 28-day specified compressive strength \( f' \) of not less than 2,500 psi (17.24 MPa). Where concrete is placed through a funnel hopper at the top of the pier, the concrete mix shall be designed and proportioned so as to produce a cohesive workable mix having a slump of not less than 4 inches (102 mm) and not more than 6 inches (152 mm). Where concrete is to be pumped, the mix design including slump shall be adjusted to produce a pumpable concrete.

1811.4 Reinforcement. Except for steel dowels embedded 5 feet (1524 mm) or less in the pier, reinforcement where required shall be assembled and tied together and shall be placed in the pier hole as a unit before the reinforced portion of the pier is filled with concrete.

Exception: Reinforcement is permitted to be wet set and the 2 ⅜ inch (64 mm) concrete cover requirement be reduced to 2 inches (51 mm) for Group R, Division 3 and Group U, Division 1 occupancies not exceeding two stories of light-frame construction, provided the construction method can be demonstrated to the satisfaction of the building official.

Reinforcement shall conform to the requirements of Sections 1809.1.2.1 and 1809.1.2.2

Exceptions:
1. Isolated piers supporting posts of Group R, Division 3 and Group U, Division 1 occupancies not exceeding two stories of light-frame construction is permitted to be reinforced as required by rational analysis but not less than a minimum of one No. 4 bar, without ties or spirals, when detailed so the pier is not subject to lateral loads and the soil is determined to be of adequate stiffness.
2. Isolated piers supporting posts and bracing from decks and patios appurtenant to Group R, Division 3 and Group U, Division 1 occupancies not exceeding two stories of light-frame construction may be reinforced as required by rational analysis but not less than one No. 4 bar, without ties or spirals, when the lateral load, \( E \), to the top of the pier does not exceed 200 pounds (890 N) and the soil is determined to be of adequate stiffness.
3. Piers supporting the concrete foundation wall of Group R, Division 3 and Group U, Division 1 occupancies not exceeding two stories of light-frame con-
struction is permitted to be reinforced as required by rational analysis but not less than two No. 4 bars, without ties or spirals, when it can be shown the concrete pier will not rupture when designed for the maximum seismic load $E_m$, and the soil is determined to be of adequate stiffness.

4. Closed ties or spirals where required by Section 1809.1.2.2, are permitted to be limited to the top 3 feet (914 mm) of the piers 10 feet (3048) or less in depth supporting Group R, Division 3 and Group U, Division 1 occupancies of Seismic Design Category D, not exceeding two stories of light-frame construction.

### 1811.5 Concrete placement

Concrete shall be placed in such a manner as to ensure the exclusion of any foreign matter and to secure a full-sized shaft. Concrete shall not be placed through water except where a tremie or other approved method is used. When depositing concrete from the top of the pier, the concrete shall not be chuted directly into the pile but shall be poured in a rapid and continuous operation through a funnel hopper centered at the top of the pier.

### 1811.6 Belled bottoms

Where pier foundations are belled at the bottom, the edge thickness of the bell shall not be less than that required for the edge of footings. Where the sides of the bell slope at an angle less than 60 degrees (1 rad) from the horizontal, the effects of vertical shear shall be considered.

### 1811.7 Masonry

Where the unsupported height of foundation piers exceeds six times the least dimension, the allowable working stress on piers of unit masonry shall be reduced in accordance with ACI 530/ASCE 5/TMS 402.

### 1811.8 Concrete

Where adequate lateral support is not provided, and the unsupported height to least lateral dimension does not exceed three, piers of plain concrete shall be designed and constructed as pilasters in accordance with ACI 318. Where the unsupported height to least lateral dimension exceeds three, piers shall be constructed of reinforced concrete, and shall conform to the requirements for columns in ACI 318.

**Exception:** Where adequate lateral support is furnished by the surrounding materials as defined in Section 1807.2.9, piers are permitted to be constructed of plain or reinforced concrete. The requirements of ACI 318 for bearing on concrete shall apply.

### 1811.9 Steel shell

Where concrete piers are entirely encased with a circular steel shell, and the area of the shell steel is considered reinforcing steel, the steel shall be protected under the conditions specified in Section 1807.2.17. Horizontal joints in the shell shall be spliced to comply with Section 1807.2.7.

### 1811.10 Dewatering

Where piers are carried to depths below water level, the piers shall be constructed by a method that will ensure accurate preparation and inspection of the bottom, and the depositing or construction of sound concrete or other masonry in the dry.